

## EHTC 2010

# Impact of concrete structures

## *Reinforced Slabs and Beams*

*Presentors : Gael LAUMOND / Sylvain THOLANCE*

*Company : ATR Ingénierie*

*119 Bd STALINGRAD, 69100 VILLEURBANNE (France)*

*Contact : standard +33 (0) 4 78 94 32 02*

*[gael.laumond@atr-ingenierie.fr](mailto:gael.laumond@atr-ingenierie.fr) / [sylvain.tholance@atr-ingenierie.fr](mailto:sylvain.tholance@atr-ingenierie.fr)*

- ❖ **Goals of the study**
  
- ❖ **Drop Test on a slab**
  - ❖ **Geometry of the slab**
  - ❖ **Presentation of FE models**
  - ❖ **Simulation Results**
  
- ❖ **Conclusions**
  
- ❖ **Perspectives**

- ❖ **Define the right modelling of reinforced concrete slab perforation under RADIOSS.**
  
- ❖ **Identificate important variables:**
  - ❖ **Influence of concrete law**
  - ❖ **Influence of elements and size mesh**
  - ❖ **Influence of reinforcement modelling**
  
- ❖ **Comparison with tests of Heriot Watt University performed by Prof I. M. May et al. and with others calculations under LS DYNA.**

## Geometry of the slab

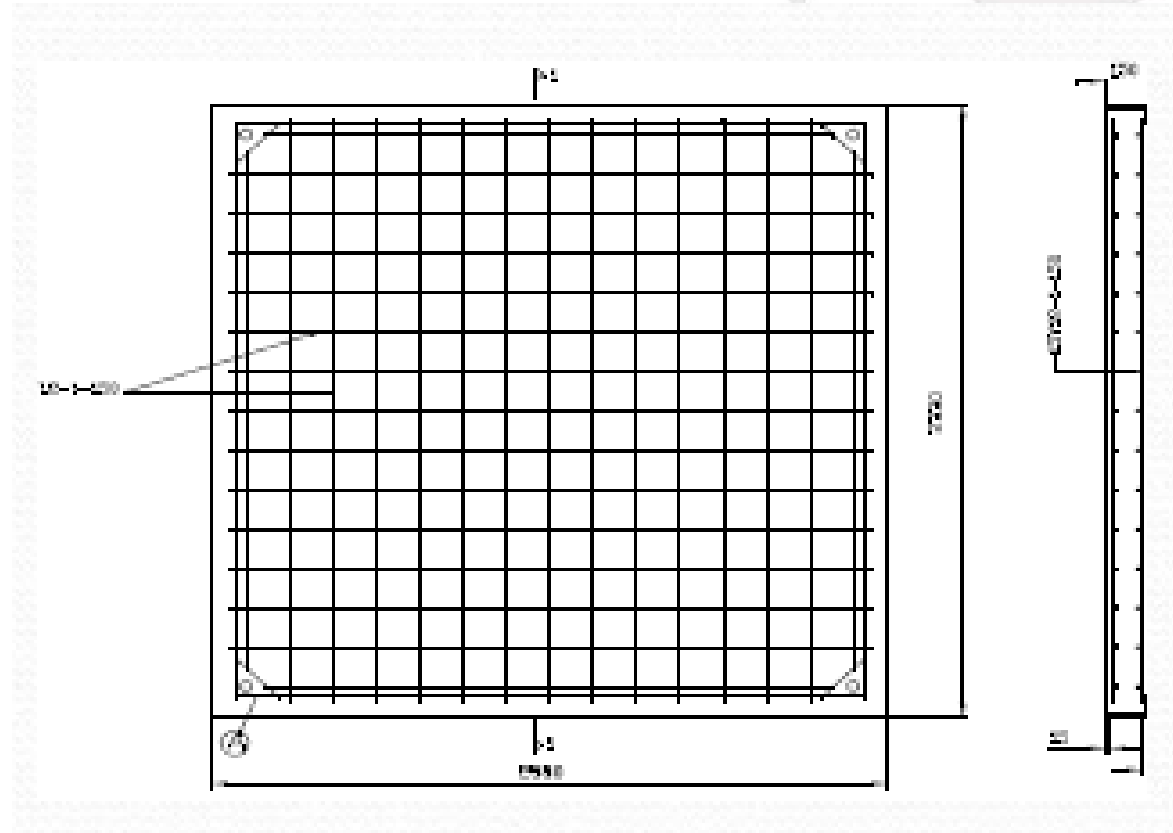
Square reinforced concrete slab in a U-type steel frame maintained at each corner

External dimensions :

- Length = 2,33 m
- Width = 2,33 m
- Height = 0,15 m

Reinforcement :

- 2 layers
- Ø12 150 x 150 mm



## Presentation of the slabs

Three different modellings are tested :

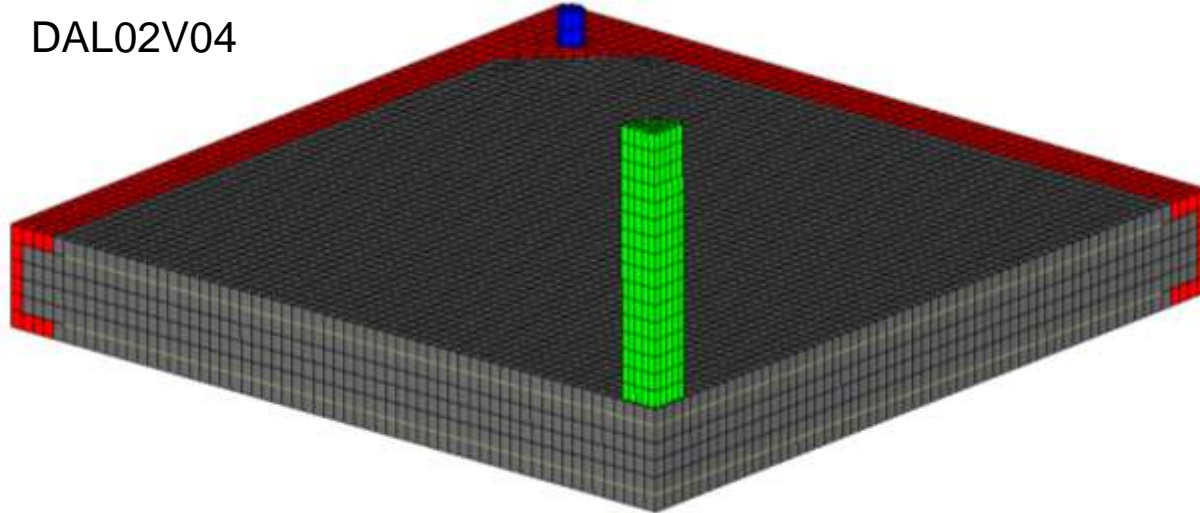
- Large meshing size
- Medium meshing size
- Small meshing size

Within these modellings, the influence of the reinforcement modelling is explored :

- 1D common-node with the concrete
- 3D common-node with the concrete
  - Reinforcement grid in a single plan
  - Reinforcement grid with neutral fiber gap

## Presentation of the large meshing-size slab S10

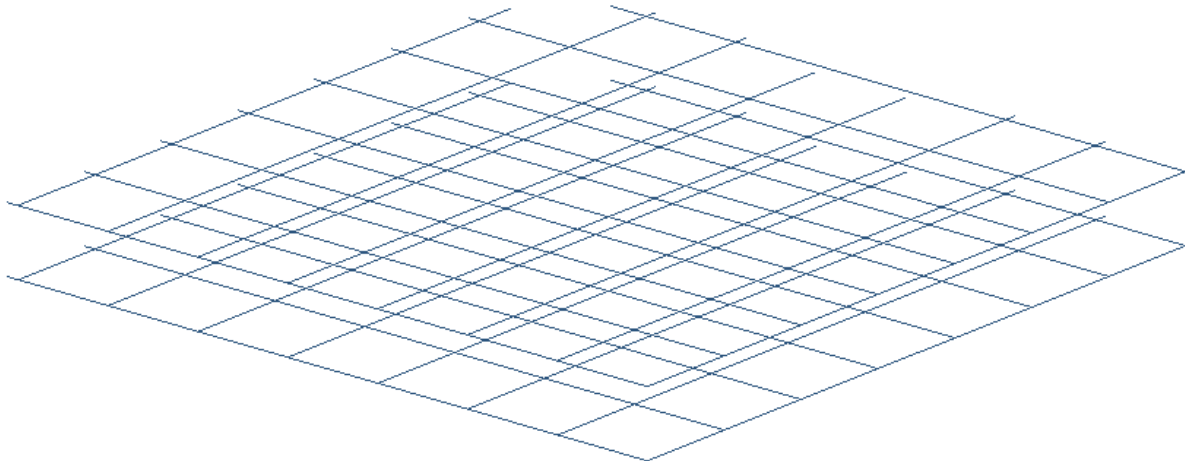
DAL02V04



Size : ~ 25 mm

25 316 elements

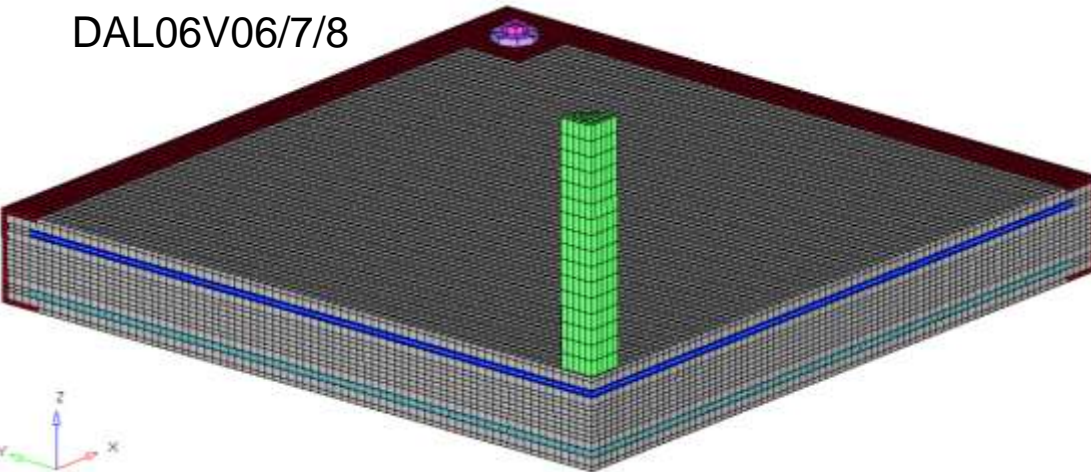
Reinforcement : Beam CN\*



\* CN = Common nodes

## Presentation of the medium meshing-size slab S2i

DAL06V06/7/8



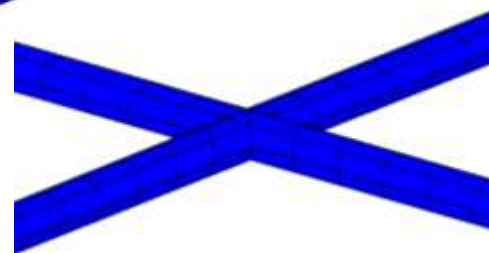
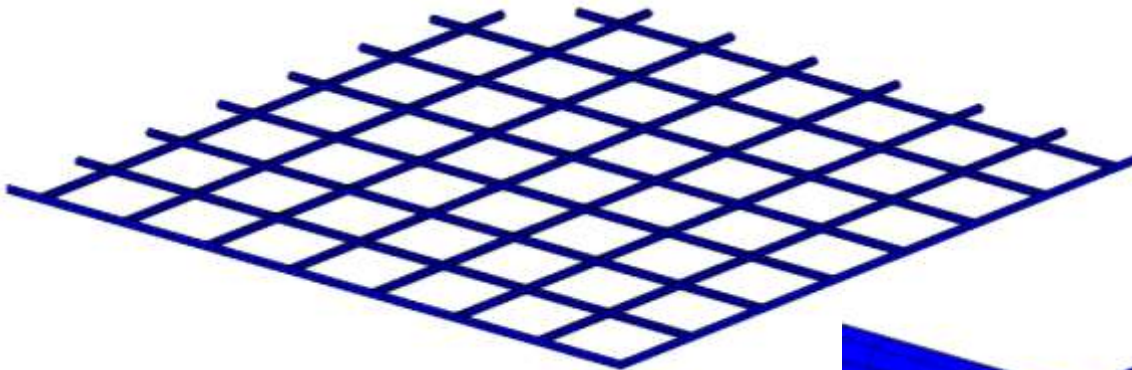
Size : ~ 10 mm

139 960 elements

**S21** Reinforcement : Beam CN\*

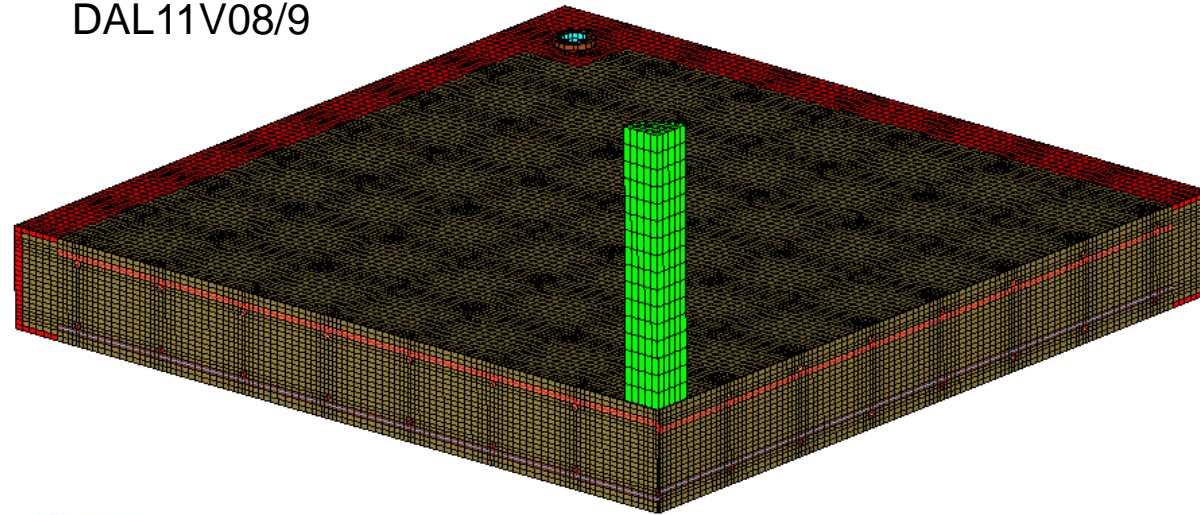
**S22** Reinforcement : Solid CN\*

**S23** Without Reinforcement



## Presentation of the small meshing-size slab S3i

DAL11V08/9

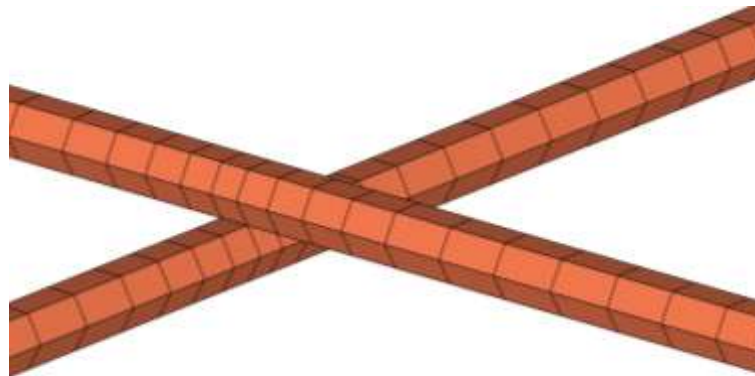
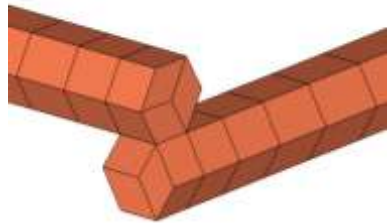


Size : ~ 6 mm

316 890 elements

**S31** Reinforcement : Beam CN\*

**S32** Reinforcement : Solid CN\*



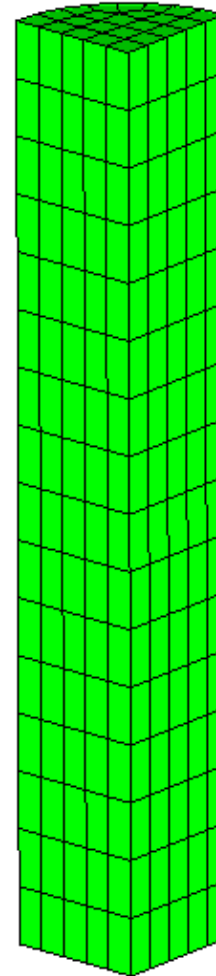
## Impactor definition

Material : Steel

Diameter : 120 mm

Mass : 380 kg (full)

Impact Velocity : 8,3 m/s (3,5 m drop off)



## Materials

Steels : /MAT/PLAS\_JOHNS

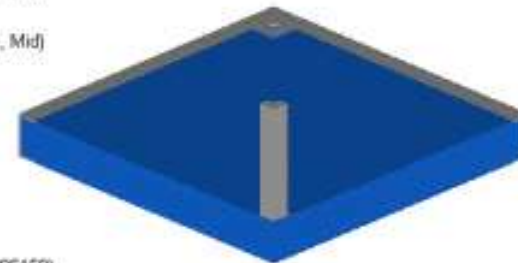
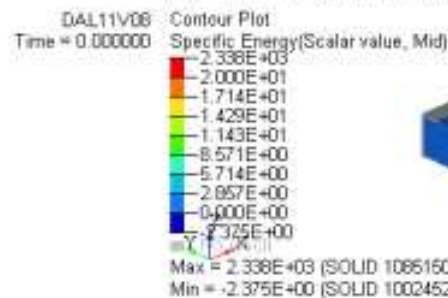
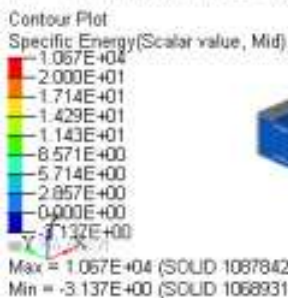
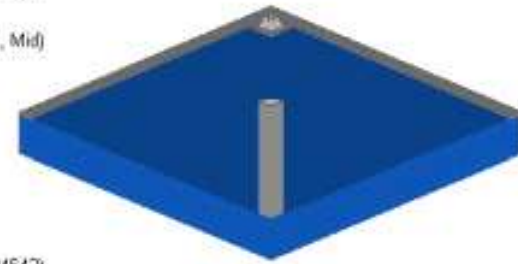
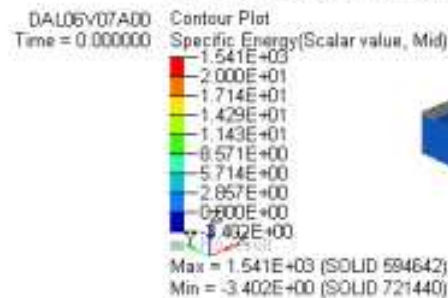
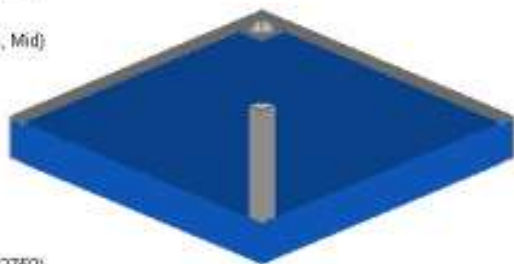
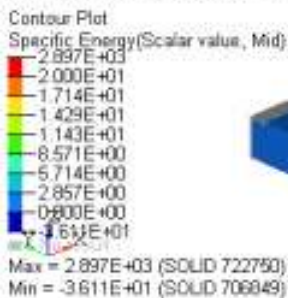
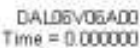
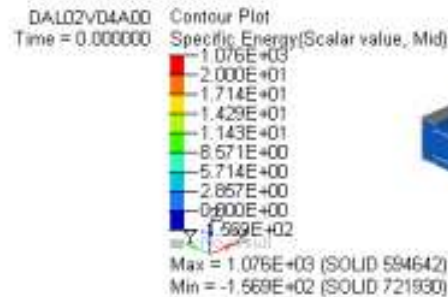
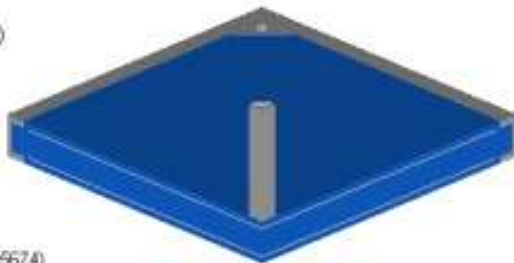
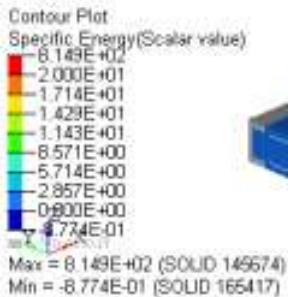
	Frame / Impactor	Reinforcement
$\rho$ (kg/m <sup>3</sup> )	7850	7850
E (GPa)	210	210
$\nu$	0,3	0,3
$\sigma_e$ (MPa)	235	500
$\sigma_{max}$ (MPa)	340	640

Concrete : /MAT/CONC

	Concrete
$\rho$ (kg/m <sup>3</sup> )	2400
E (GPa)	40
$\nu$	0,2
$f_c$ (MPa)	60

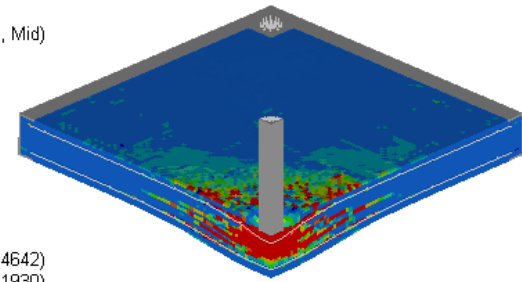
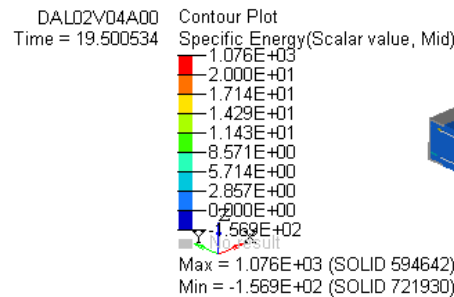
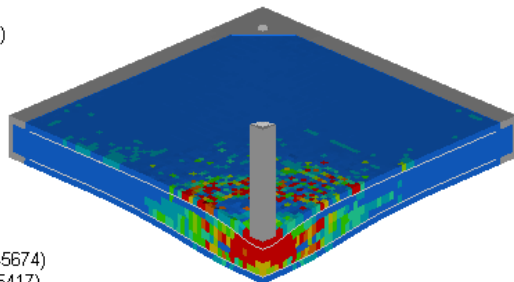
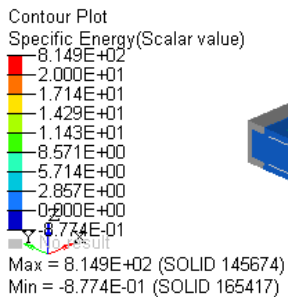
### Simulation Results

### Evolution of Specific Energy (mJ)

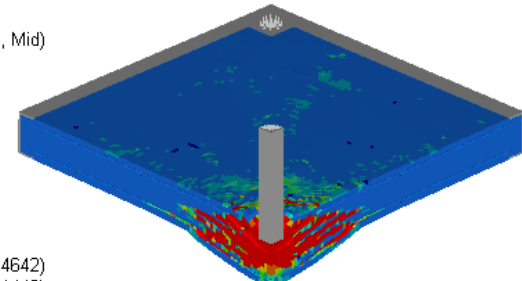
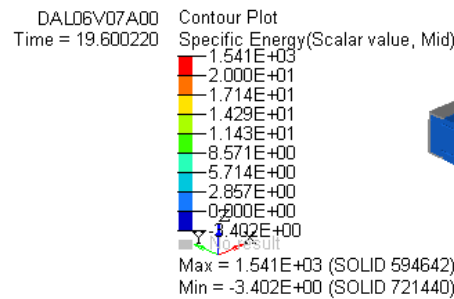
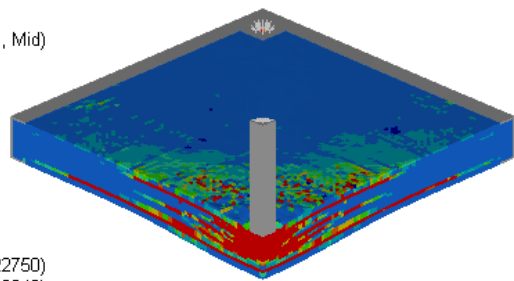
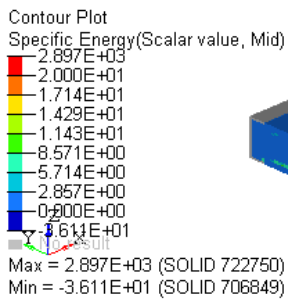


### Simulation Results

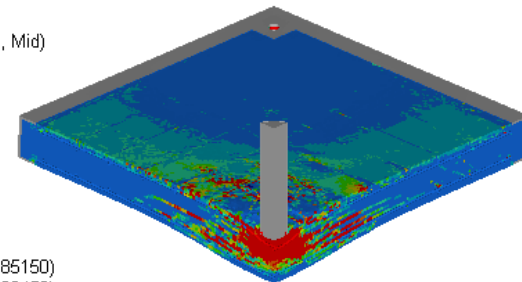
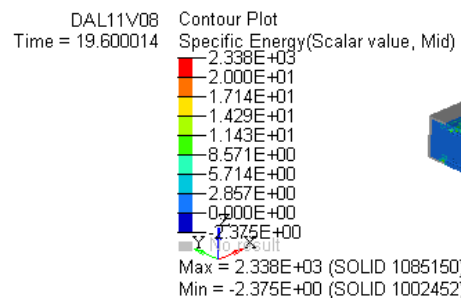
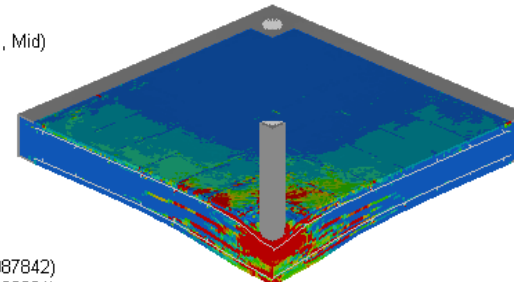
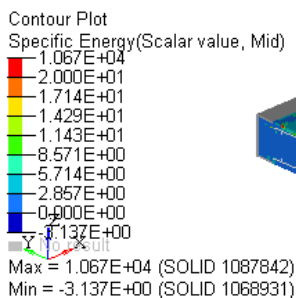
### Specific Energy at $t \approx 20$ ms (mJ)



DAL06V06A00  
Time = 19.600033



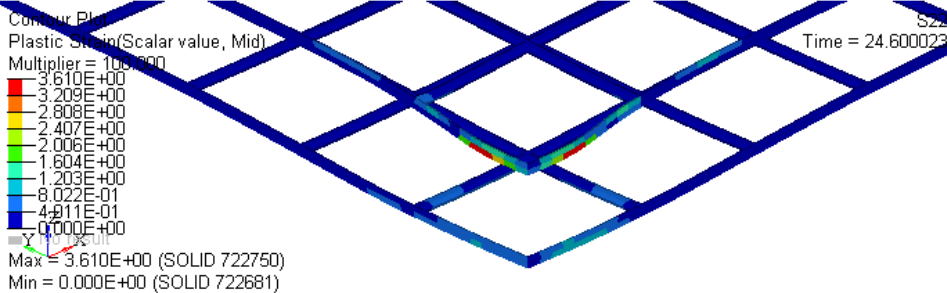
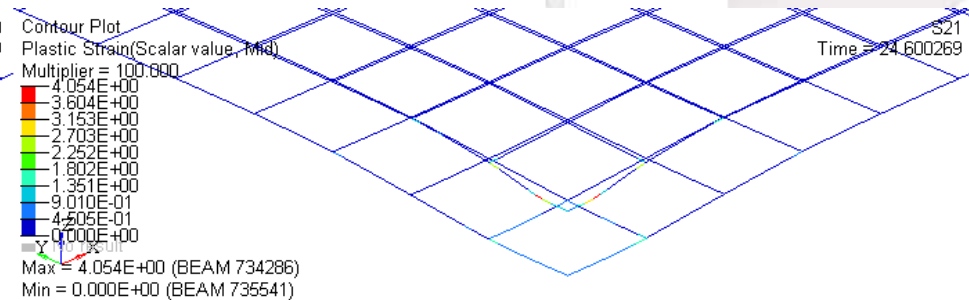
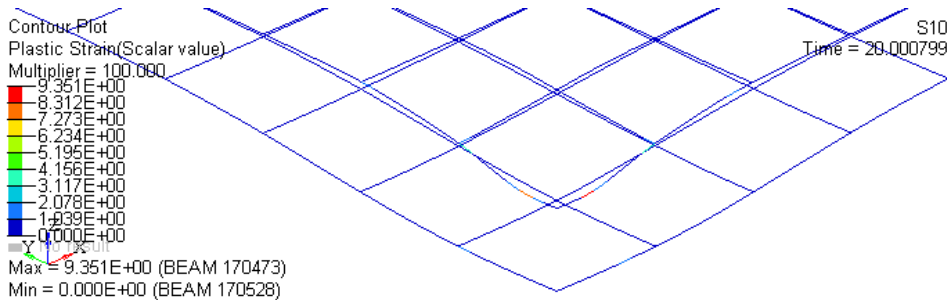
DAL06V08A00  
Time = 19.600210



DAL11V09  
Time = 19.600006

### Simulation Results

Plastic Strain of the reinforcement at  $t \approx 20$  ms (%)



**S10 : 9,35 %**

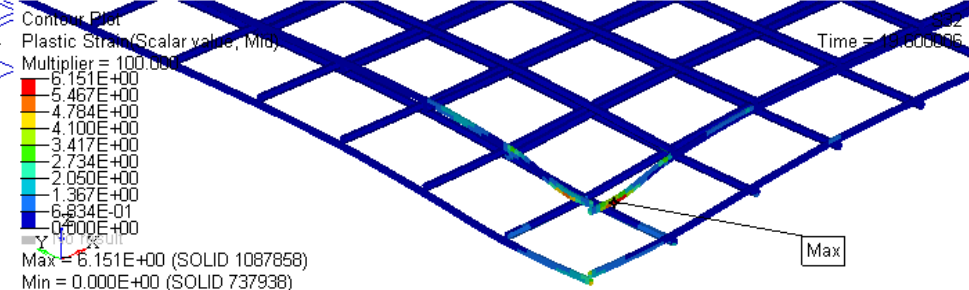
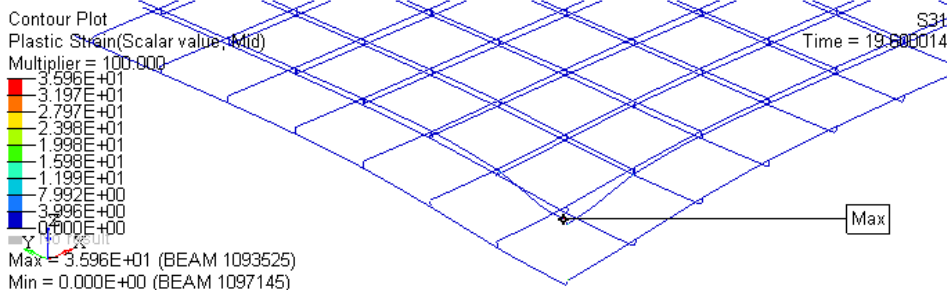
**S21 : 4,05 %**

**S22 : 3,61 %**

**S23 : ----**

**S31 : 3,60 %**

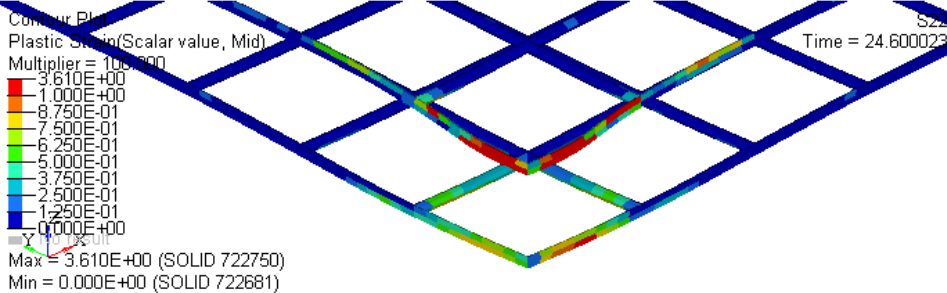
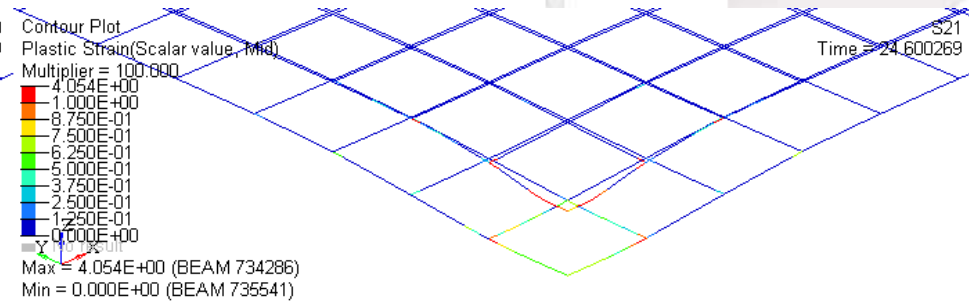
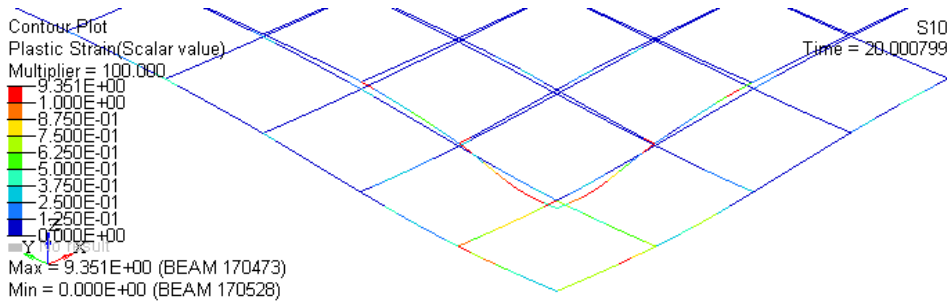
**S32 : 6,15 %**



S23 : ----  
Time = 19.600210

### Simulation Results

Plastic Strain of the reinforcement at  $t \approx 20$  ms (max 1 %)



**S10 : 9,35 %**

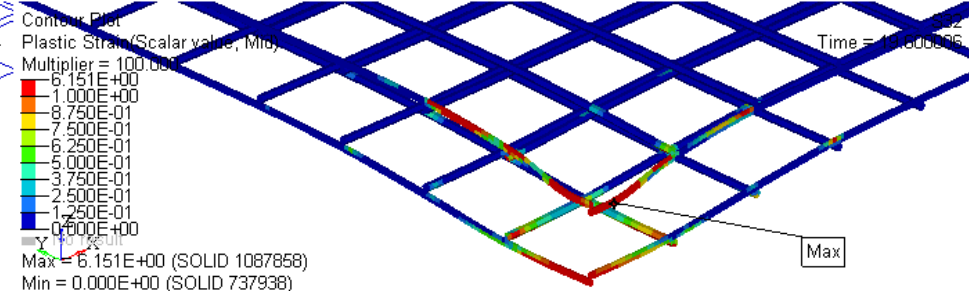
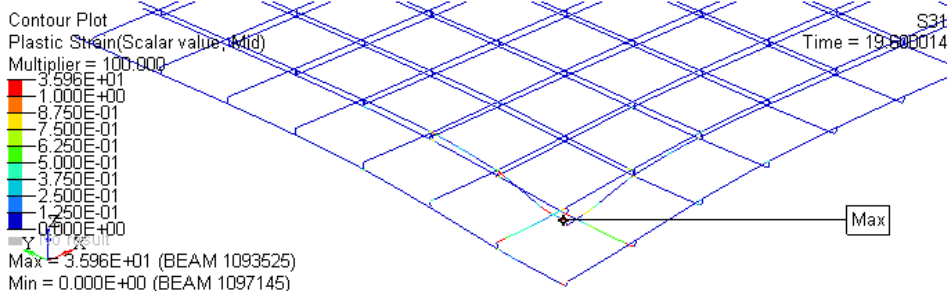
**S21 : 4,05 %**

**S22 : 3,61 %**

**S23 : ----**

**S31 : 3,60 %**

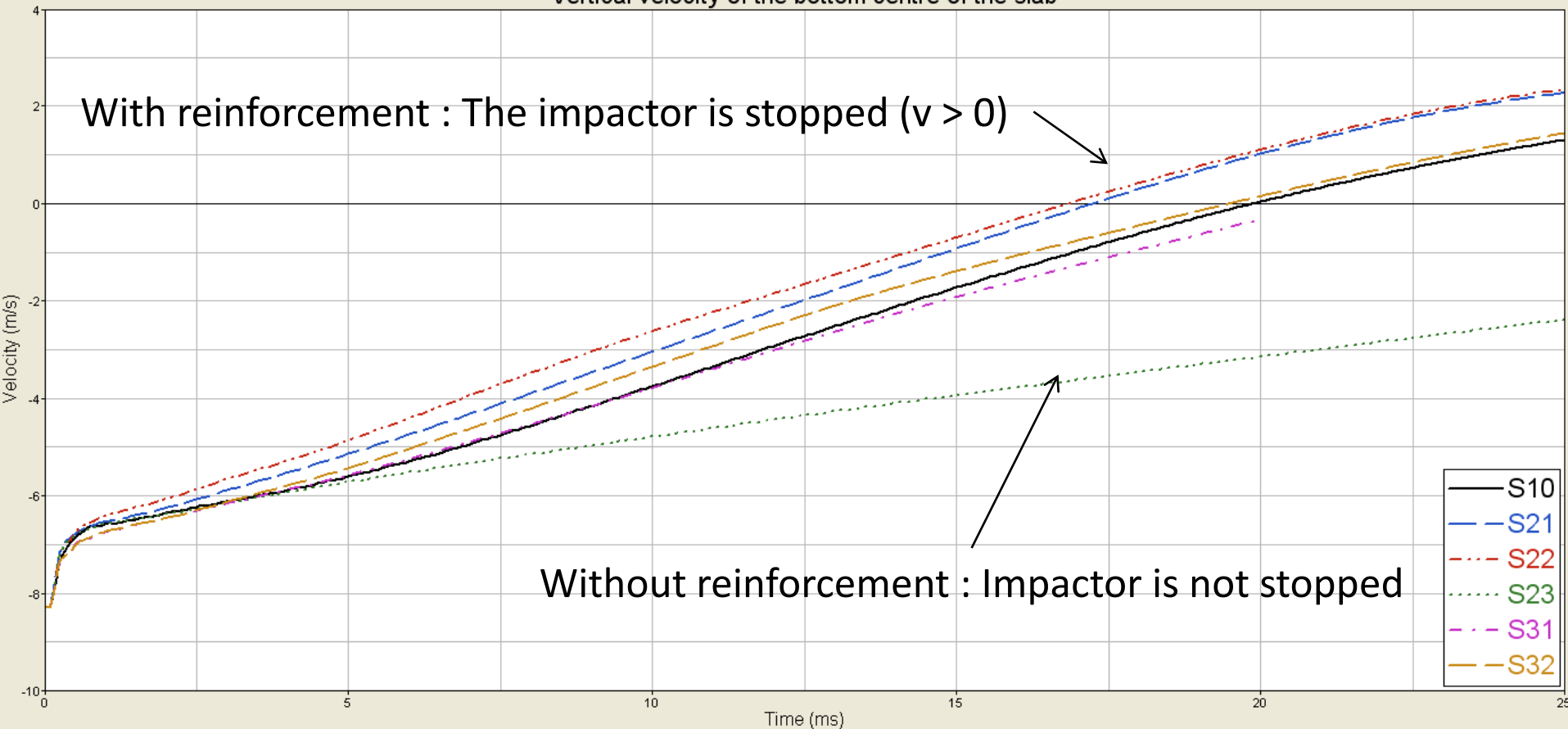
**S32 : 6,15 %**



### Simulation Results

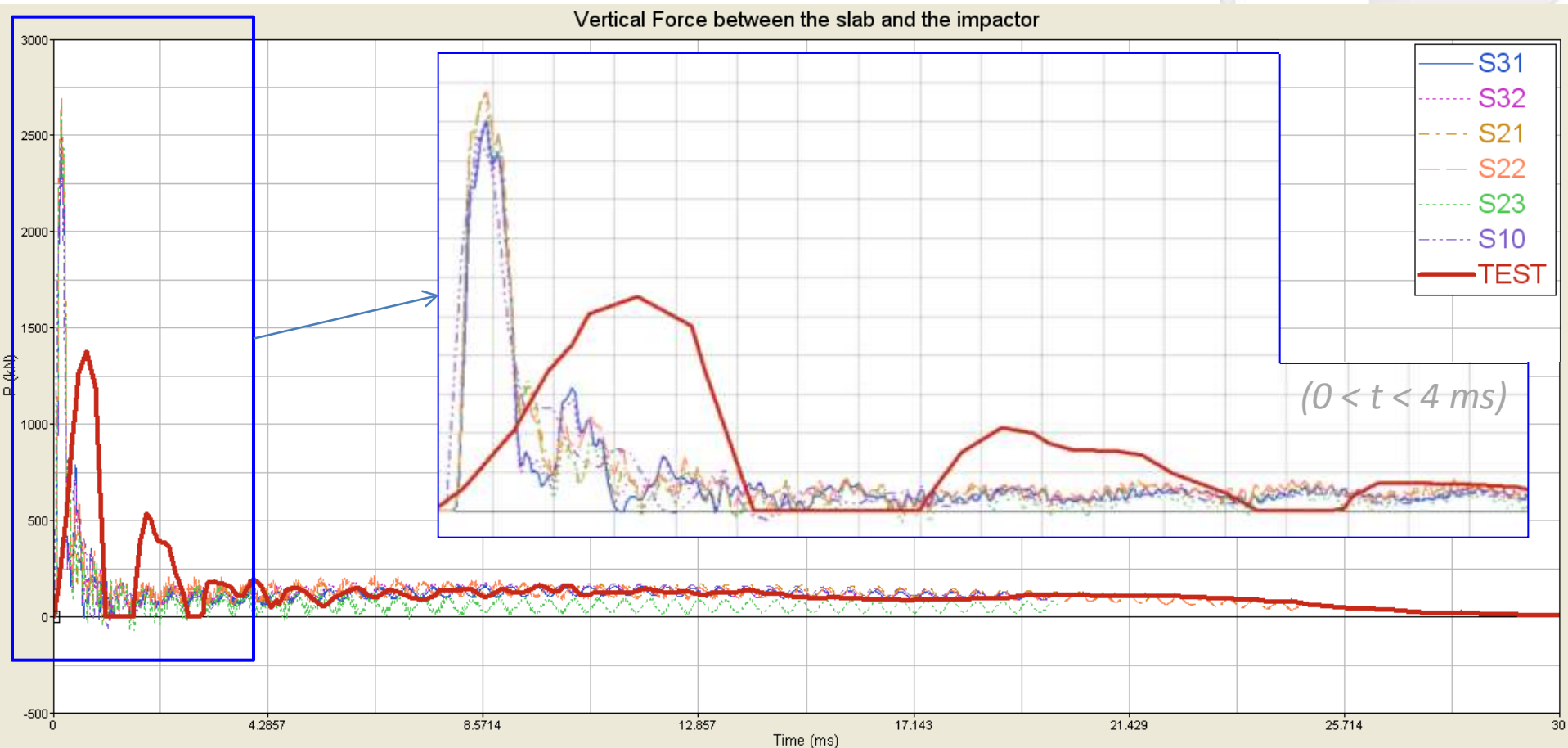
#### Velocity of the impactor

Vertical velocity of the bottom centre of the slab



### Simulation Results

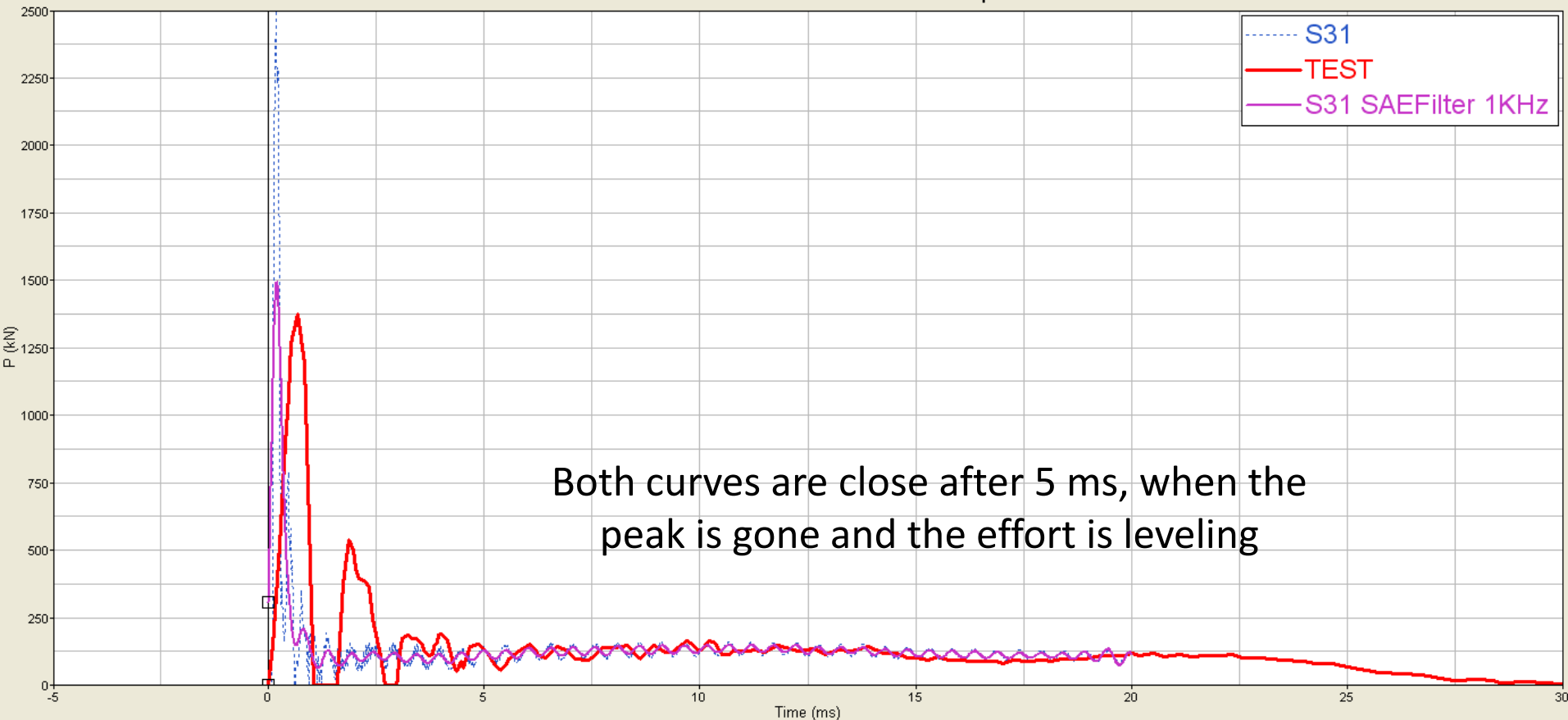
Force between the impactor and the slab (kN)



### Simulation Results

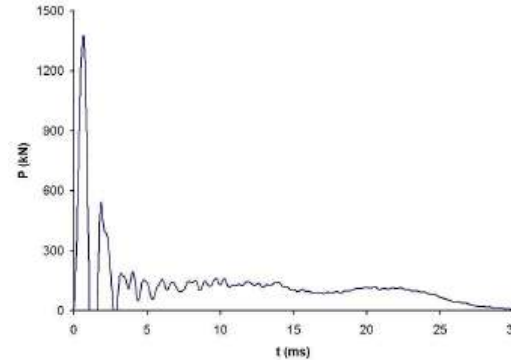
Force between the impactor and the slab (kN)

Vertical Force between the slab and the impactor



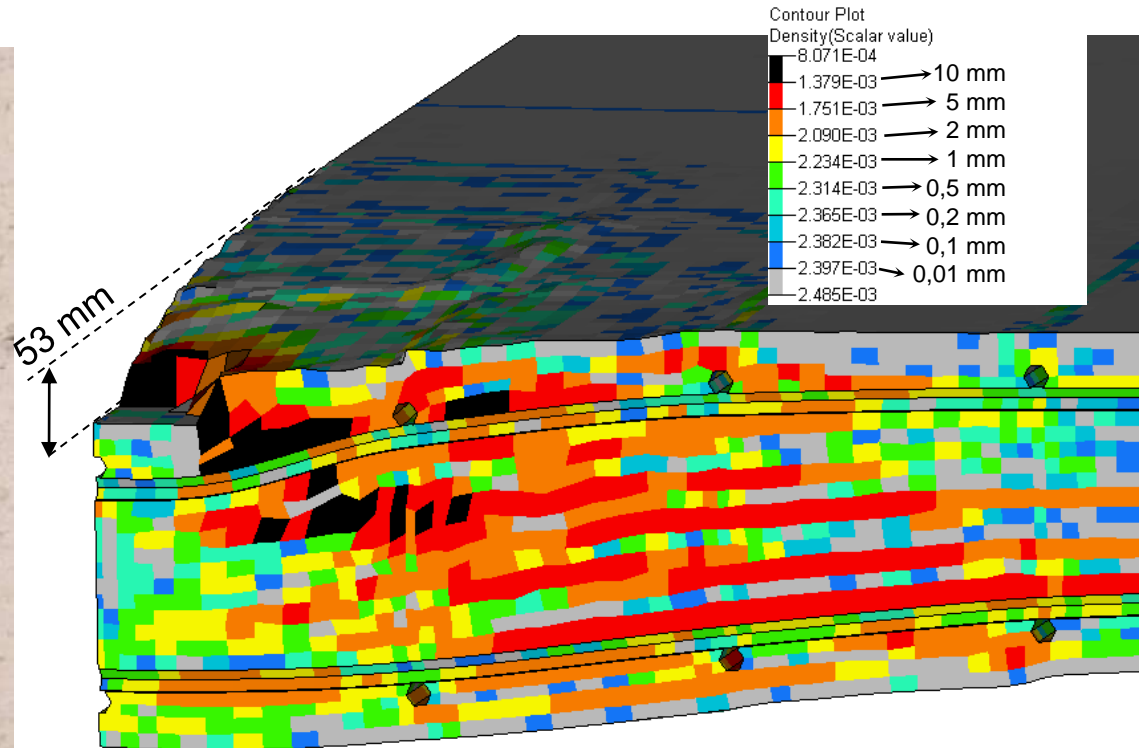
Both curves are close after 5 ms, when the peak is gone and the effort is leveling

## Drop Test Installation



## Simulation Results

Estimation of the cracking of slab S31 – comparison with test  
*Top face*

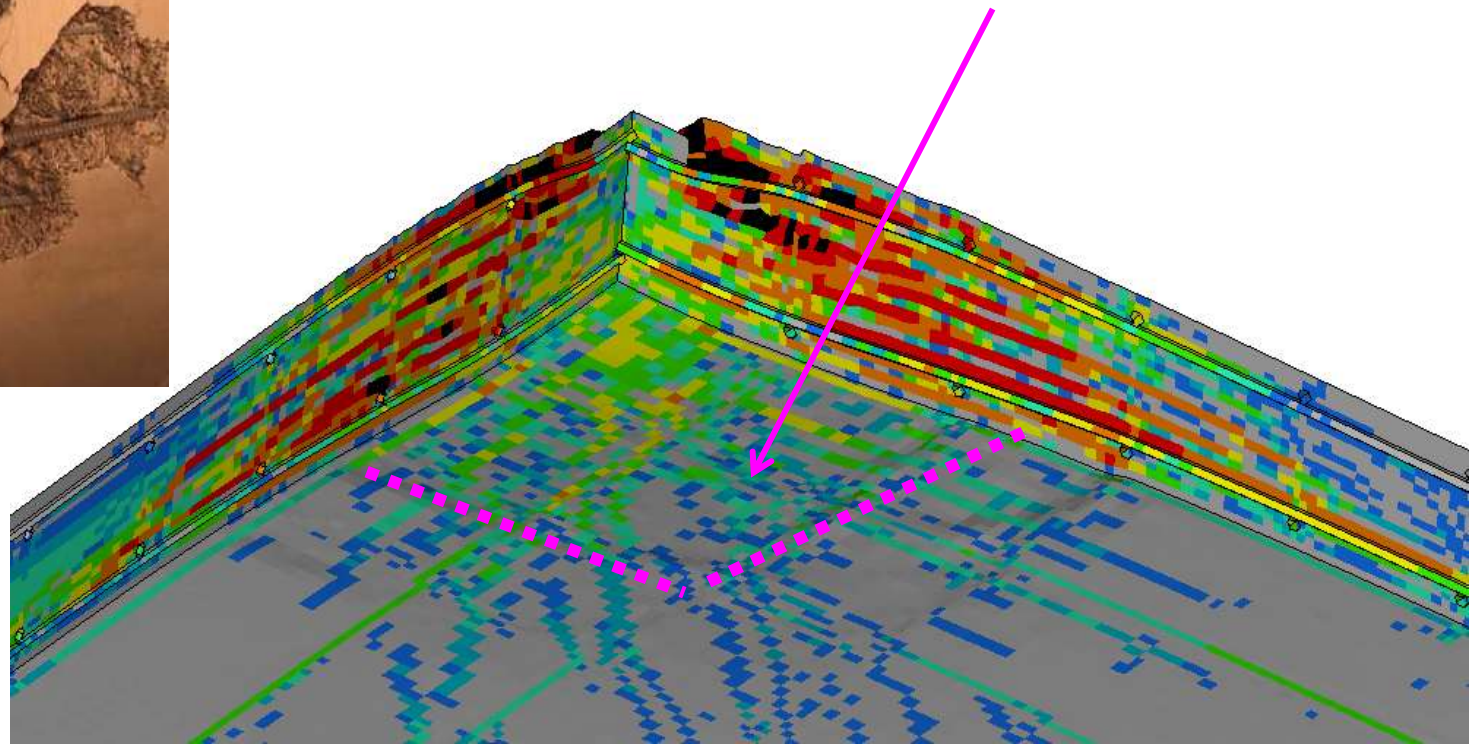


## Simulation Results

Estimation of the cracking of slab S31 – comparison with test  
*Bottom face*

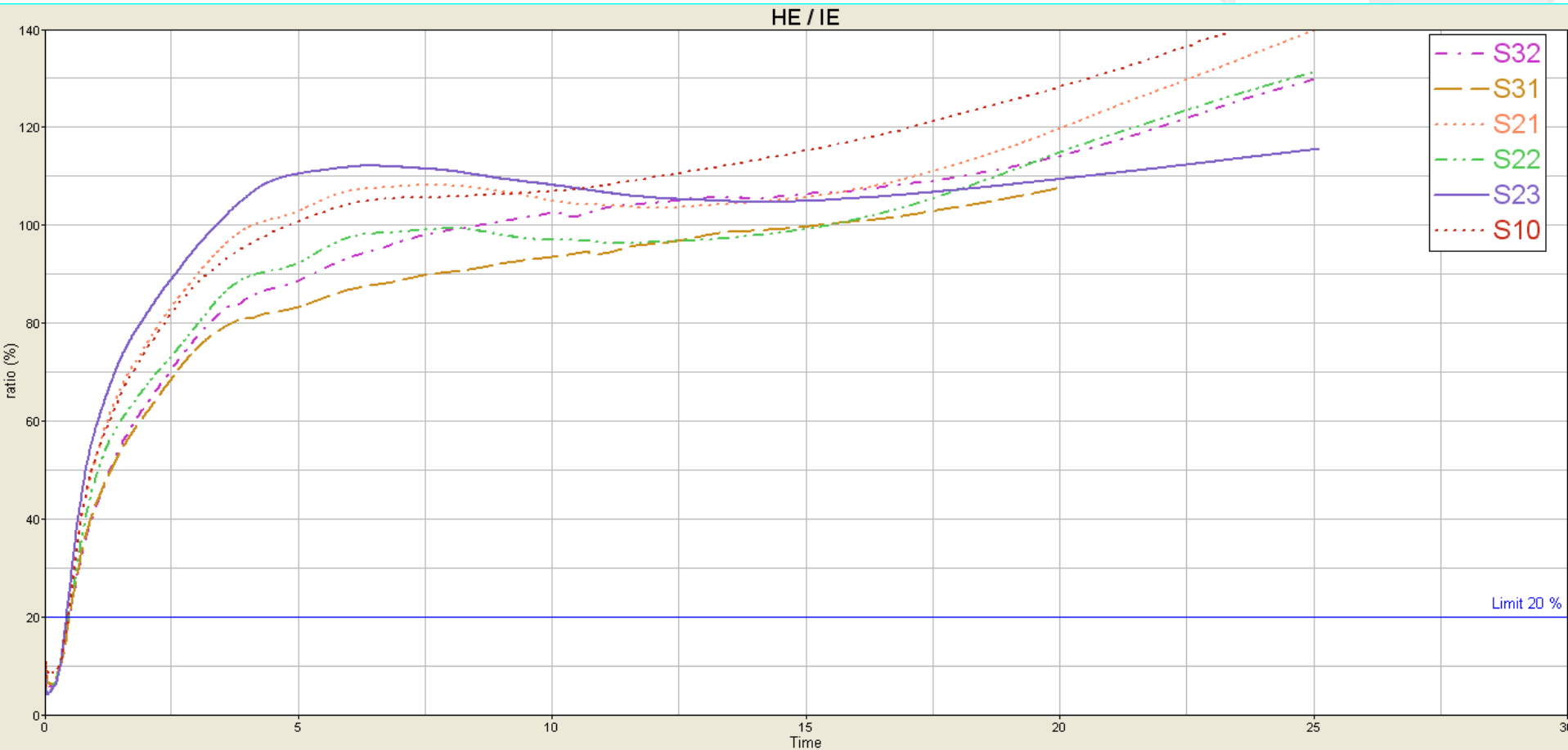


Area of massive cracks in the test



### Simulation Results

Comparison between Hourglass and Internal Energies



### Conclusions of the impact simulation on a slab

Whatever the modelling is, global variables of the problem stay almost the same. The simulation enables the user to see the cracking cone inside the thickness of the slab and its propagation along the reinforcement.

The maximum plastic deformation is in the same range for a 1D or a 3D modelling even if the neutral gap can increase a bit the deformation on the external wires.

Although the Hourglass Energy is really important ( $> IE$ ), the model seems to be close to the real test in terms of vertical force and cracking, except at the beginning of the impact when the rigidity is really different.

This sequence should probably be influenced by the structure above the slab during the test.

Other tests should be runned on simple structure to improve the knowledge like reinforced beams.

**Perform calculations under simpler structures and smaller models to make many iterations**

**Use other elements like TETRA and SPH**

**Comparison with LS DYNA results using other concrete laws**

**APPENDIX 1: Crack opening assessment by density contours**

## Simulation Results

In order to estimate the cracking, we are using the density contour. Mostly, cracking appears where the structure is in traction. Considering an element can be in traction 1 to 3 directions, and neglecting the deformations on the other directions (Poisson effects), the volume variation is directly linked to potential of cracking.

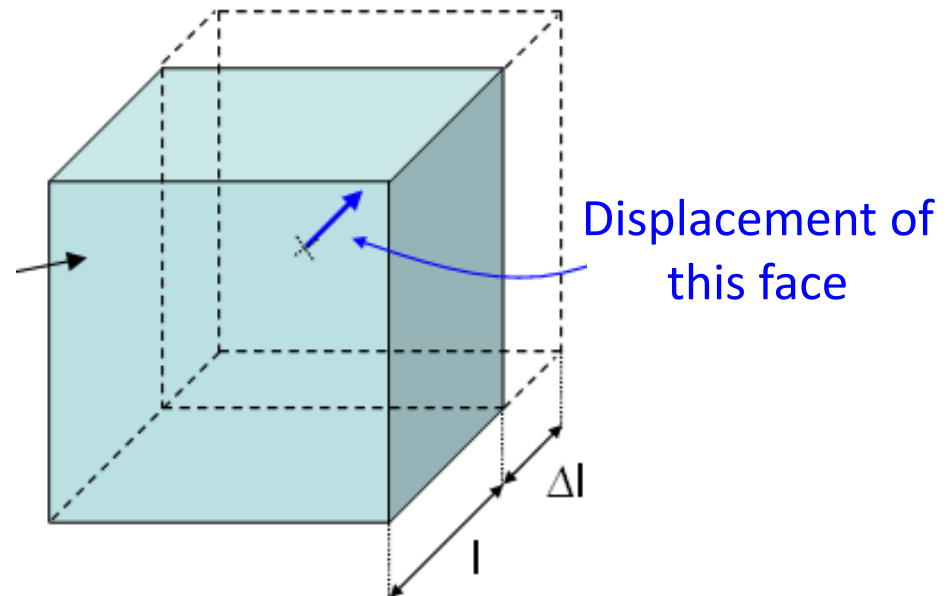
Example & Hypothesis for uniaxial traction :

Elements as perfect cubes  
(length = width = height = L)

Initial density = 2400 kg/m<sup>3</sup>

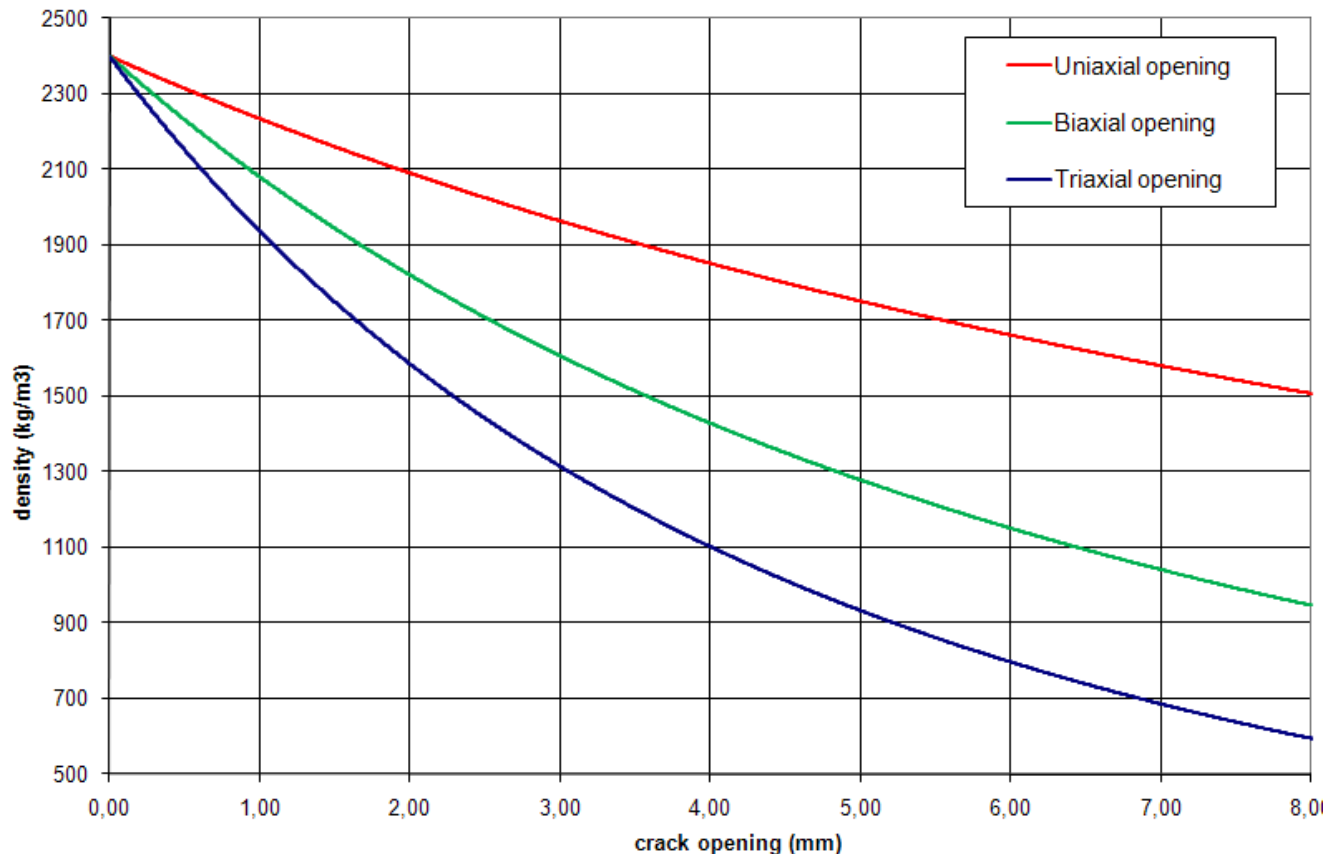
Initial volume =  $L^3$

Final volume =  $(L + \Delta L)^3$



## Simulation Results

For the 3 kinds of solicitations, the crack opening can be estimate by the density contour, considering the uniaxial opening as the worst for a fixed density.



**APPENDIX 2: Drop tests on reinforced beams**

## DOE of drop tests on reinforced beams:

### Variables:

*Beam modelling: 8 types*

*2 impactor shapes : cylinder and hemispheric*

*Concrete properties:  $f_c = 32, 60, 90 \text{MPa}$*

*Impactor speed: 2, 5 and 8 m/s*

⇒ **Complete DOE leads to 144 calculations**

### Responses:

*max of force, acceleration and displacement,*

*Speed at simulation end*

*Ratio Hourglass energy on Internal energy (HE/IE)*

*Max plastic deformation of reinforcement*

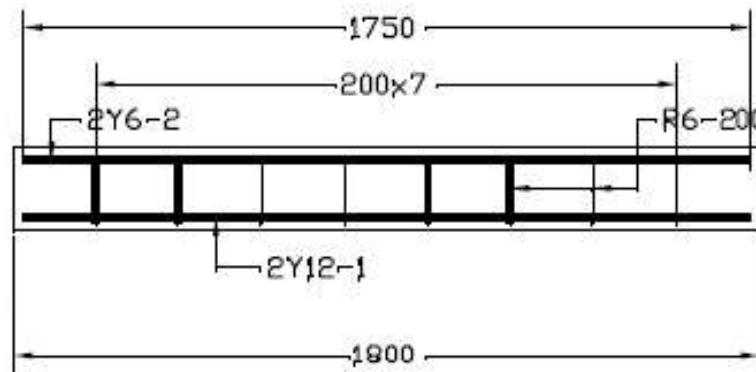
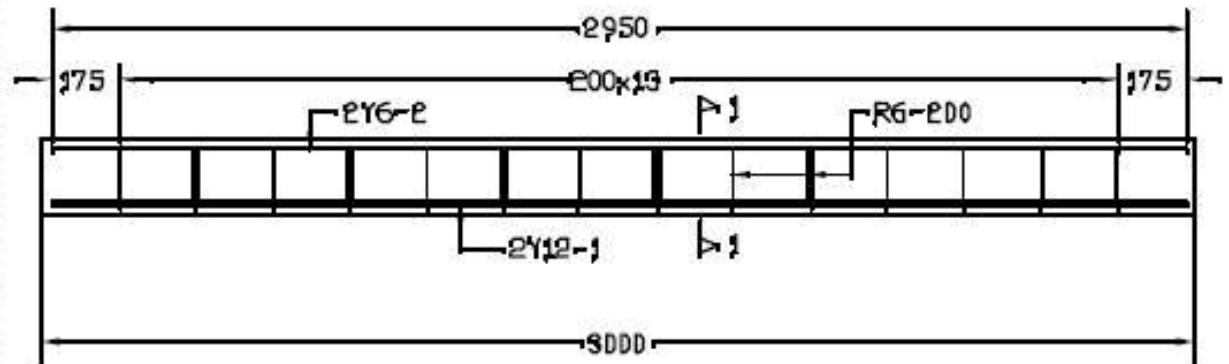
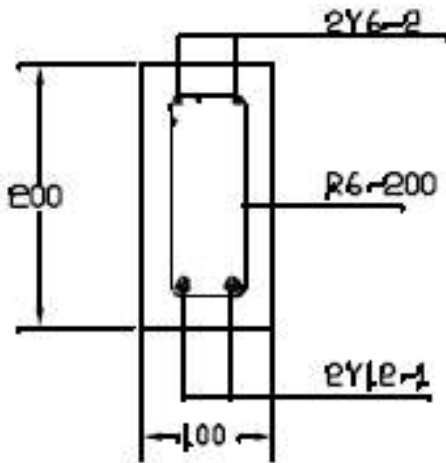
### Tools and analysis:

*All calculations made with RADIOSS*

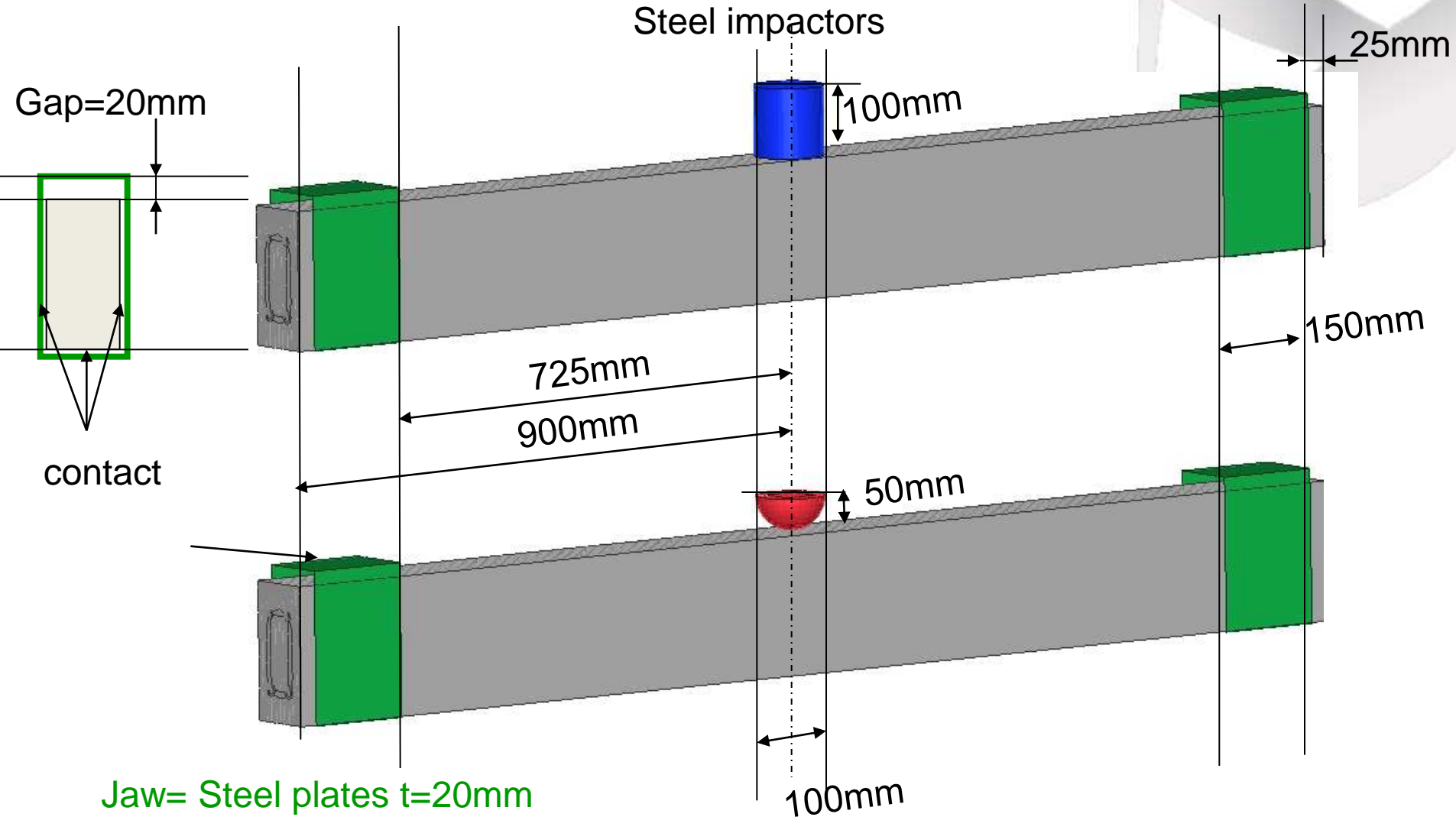
*DOE built and analysed with HyperstudyDSS*

## Two geometries: one section and two lengths

20mm COVER TO MAIN STEEL



**Loadcase and boundary conditions**



### Beams modelling:

**0.** Direct in the law n°24: volumic percentage of steel /concrete by direction

**1.** No reinforcement: only concrete

**2.** Reinforcement modeled by brick elements: common nodes between steel and concrete

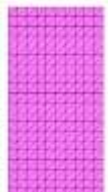
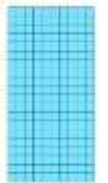
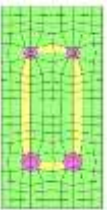
**3.** Reinforcement modeled by beam elements: common nodes between steel and concrete

**4.** Reinforcement modeled by beam elements fixed on concrete bricks by a type 2 interface

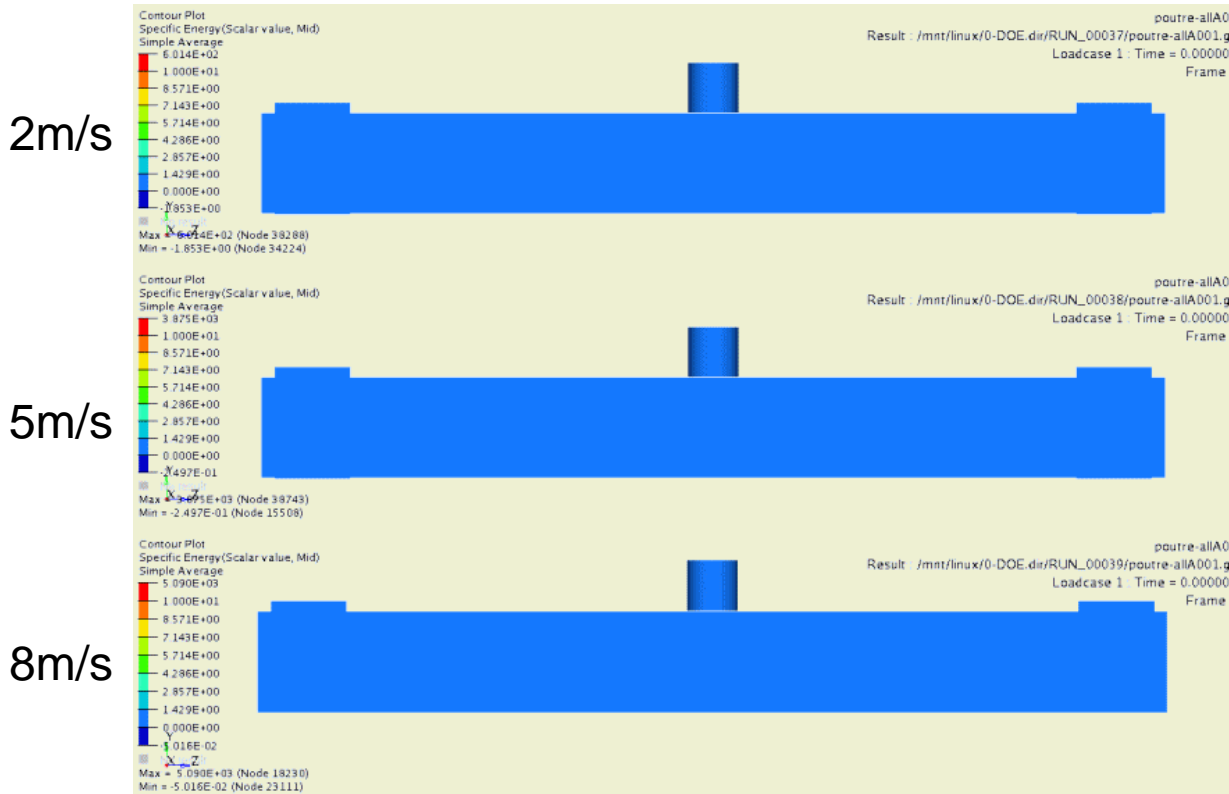
**5.** Perfect Brick mesh + reinforcement modeled by beam elements: common nodes between steel and concrete

**6.** TETRA mesh + reinforcement modeled by beam elements fixed on concrete bricks by a type 2 interface

**7.** SPH mesh for concrete + Reinforcement modeled by bricks elements. SPH are fixed on concrete bricks by a type 2 interface



### Animation: contour specific energy (mJ/g)



- Punching at the top face
- Oblique cracks
- Propagation of cracks near the bottom reinforcement
- Beam bending
- Concrete burst at the bottom face

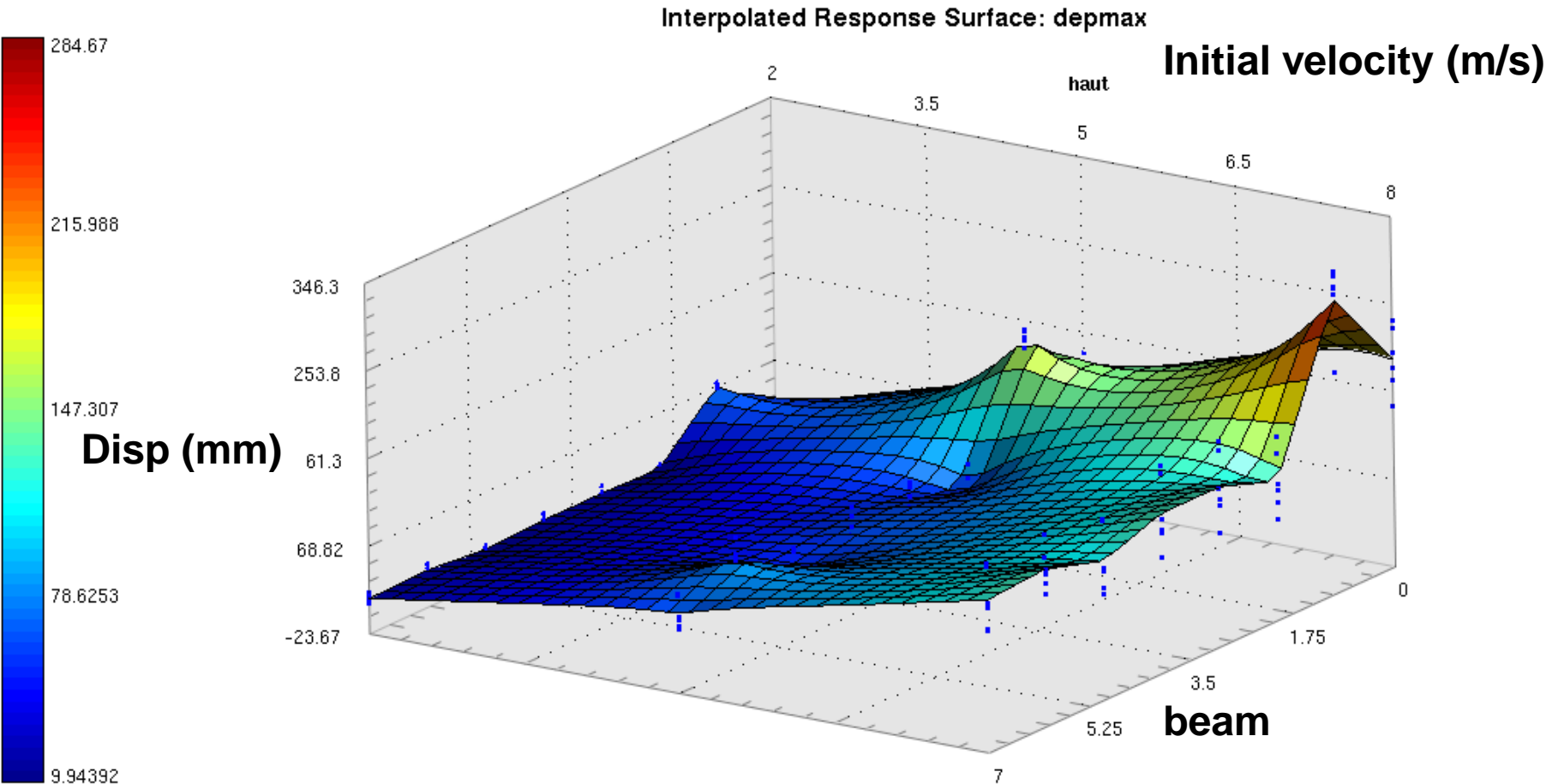
Impactor= cylinder

BEAM 2: all in bricks: concrete and reinforcement (Isolid=1 and lframe=2)

Concrete  $f_c=32\text{MPa}$

 Response surface :

Impactor max displacement=f(beam, initial velocity)



**Response surface of  $HE/IE=f(\text{beam}, \text{initial velocity})$**

